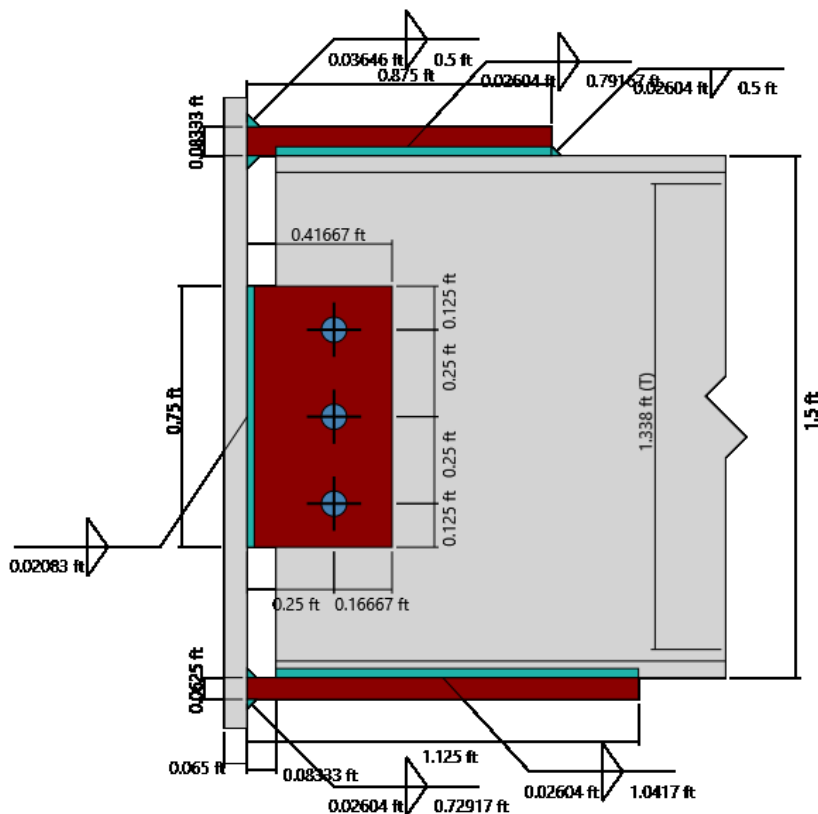


## Design Summary: Welded Flange Plate



### Top Plate Design:

Length = 0.875 ft  
 Width = 0.5 ft  
 Thickness = 0.08333 ft  
 $F_y = 7200 \text{ Ksf}$   
 $F_u = 9360 \text{ Ksf}$

### Bottom Plate Design:

Length = 1.125 ft  
 Width = 0.72917 ft  
 Thickness = 0.0625 ft  
 $F_y = 7200 \text{ Ksf}$   
 $F_u = 9360 \text{ Ksf}$

### Shear Tab Design:

Height = 0.75 ft  
 Width = 0.41667 ft  
 Thickness = 0.03125 ft  
 $F_y = 7200 \text{ Ksf}$   
 $F_u = 9360 \text{ Ksf}$

### Support Design:

Thickness = 0.065 ft  
 $F_y = 7200 \text{ Ksf}$   
 $F_u = 9360 \text{ Ksf}$

### Top Weld at Support:

Type = Double Fillet  
 $F_{exx} = 10080 \text{ Ksf}$   
 Size = 0.03646 ft  
 Length = 0.5 ft

### Bottom Weld at Support:

Type = Double Fillet  
 $F_{exx} = 10080 \text{ Ksf}$   
 Size = 0.02604 ft  
 Length = 0.72917 ft

### Shear Tab Weld Design:

Type = Double Fillet  
 $F_{exx} = 10080 \text{ Ksf}$   
 Size = 0.02083 ft  
 Length = 0.75 ft

### Beam Design:

$d = 1.5 \text{ ft}$   
 $T = 1.338 \text{ ft}$   
 $bf = 0.625 \text{ ft}$   
 $tf = 0.0475 \text{ ft}$   
 $tw = 0.02958 \text{ ft}$   
 $F_y = 7200 \text{ Ksf}$   
 $F_u = 9360 \text{ Ksf}$

### Top Weld at Flange:

Type = Double Fillet  
 End Weld? = Yes  
 $F_{exx} = 10080 \text{ Ksf}$   
 Size = 0.02604 ft

### Bottom Weld at Flange:

Type = Double Fillet  
 End Weld? = No  
 $F_{exx} = 10080 \text{ Ksf}$   
 Size = 0.02604 ft

### Shear Tab Bolt Design:

Type = Group 120  
 Diameter = 0.07292 ft  
 Thread = N  
 Spacing = 0.25 ft  
 Bolt Count = 3

### Steel Specification:

AISC 360-22

<u>Limit State</u>	<u>Load Key</u>	<u>Demand</u>	<u>Capacity</u>	<u>Unity Value</u>
<b>Shear Tab</b>				
Bolt Group Capacity	1	42 K	71.33 K	0.589
Bolt Group Capacity - Beam Web	1	42 K	73.06 K	0.575
Fillet Weld	1	42 K	100.23 K	0.419
Base Metal	1	42 K	98.719 K	0.425
Base Metal - Support	1	42 K	410.67 K	0.102
Block Shear	1	42 K	82.266 K	0.511
Shear Yield	1	42 K	101.25 K	0.415
Shear Rupture	1	42 K	65.813 K	0.638
<b>Top Flange Plate</b>				
Tension Yield	1	175 K	270 K	0.648
Tension Rupture	1	175 K	292.5 K	0.598
Compression Buckling	-	-	-	-
Block Shear	-	-	-	-
Fillet Weld - Plate to Flange	1	175 K	175.06 K	1.000
Base Metal	1	175 K	735.64 K	0.238
Base Metal - Flange	1	175 K	419.31 K	0.417
Weld - Plate to Support	1	175 K	175.41 K	0.998
Base Metal - Plate to Support	1	175 K	270 K	0.648
<b>Bottom Flange Plate</b>				
Tension Yield	-	-	-	-
Tension Rupture	-	-	-	-
Compression Buckling	1	175 K	295.12 K	0.593
Block Shear	-	-	-	-
Fillet Weld - Plate to Flange	1	175 K	174.01 K	1.006
Base Metal	1	175 K	548.44 K	0.319
Base Metal - Flange	1	175 K	416.81 K	0.420
Weld - Plate to Support	1	175 K	182.72 K	0.958
Base Metal - Plate to Support	1	175 K	399.26 K	0.438
<b>Welded Flange Plate Detailing</b>	-	-	-	OK

## Welded Flange Plate: Detailed Reports

### Tension Yield - Top Plate (AISC 360-22 J4.1.a)

Load Set: Load Set 1 Load Combination: 1.2D + 1.6L

Demand:  $R_u = 175 \text{ K}$

Capacity:  $\phi R_n = \phi \cdot F_y \cdot l \cdot t \cdot n = 0.90 \cdot 7200 \text{ Ksf} \cdot 0.5 \text{ ft} \cdot 0.08333 \text{ ft} \cdot 1$   
 $\phi R_n = 270 \text{ K}$

Unity =  $R_u / \phi R_n = 175 \text{ K} / 270 \text{ K} = \mathbf{0.648}$

### Fillet Weld - Top Plate to Flange (AISC 360-22 J2.4)

Load Set: Load Set 1 Load Combination: 1.2D + 1.6L

Demand:  $R_u = 175 \text{ K}$

Weld: Size = 0.02604 ft  $F_{exx} = 10080 \text{ Ksf}$

Longitudinal Length = 0.79167 ft Transverse Length = 0.5 ft

$R_{n\_wl} = 176.33 \text{ K}$   $R_{n\_wt} = 55.685 \text{ K}$  Weld Unit Capacity = 111.37 K/ft

$\phi R_n = \text{Phi} \cdot \text{Max}(R_{n\_wl} + R_{n\_wt}, 0.85 \cdot R_{nwl} + 1.5 \cdot R_{nwt})$  (Equation J2-6)

$\phi R_n = \text{Phi} \cdot \text{Max}(176.33 \text{ K} + 55.685 \text{ K}, 0.85 \cdot 176.33 \text{ K} + 1.5 \cdot 55.685 \text{ K})$

$\phi R_n = 175.06 \text{ K}$  (Equation J2-6)

Unity =  $R_u / \phi R_n = 175 \text{ K} / 175.06 \text{ K} = \mathbf{1.000}$

### Weld - Top Plate to Support (AISC 360-22 J2.4)

Load Set: Load Set 1 Load Combination: 1.2D + 1.6L

Demand:  $R_u = 175 \text{ K}$

Fillet Weld: Size = 0.03646 ft  $F_{exx} = 10080 \text{ Ksf}$  Weld Length = 0.5 ft Theta = 90 deg

Weld Line Count = 2 Weld Unit Capacity = 233.88 K/ft

$\phi R_n = \text{Phi} \cdot 0.6 \cdot F_{exx} \cdot (1.0 + 0.5 \cdot \sin(\text{Theta})^{1.5})$  (Equation J2-4)

$\phi R_n = 0.75 \cdot 0.6 \cdot 10080 \text{ Ksf} \cdot (1.0 + 0.5 \cdot \sin(90 \text{ deg})^{1.5})$

$\phi R_n = 175.41 \text{ K}$

Unity =  $R_u / \phi R_n = 175 \text{ K} / 175.41 \text{ K} = \mathbf{0.998}$

### Base Metal - Top Plate to Support (AISC Part 9 & 360-22 J2.4)

Load Set: Load Set 1 Load Combination: 1.2D + 1.6L

Demand:  $R_u = 175 \text{ K}$  (Tension)

Weld: Weld Length = 0.5 ft

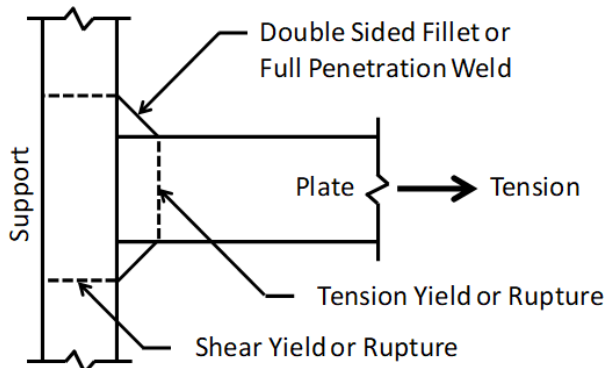


Plate:  $F_y = 7200 \text{ Ksf}$   $F_u = 9360 \text{ Ksf}$   $t = 0.08333 \text{ ft}$

Tensile Yield:  $\phi R_{n\_ty} = 0.9 \cdot F_y \cdot t \cdot \text{Weld Length} = 270 \text{ K}$  (Equation J4-1)

Tensile Rupture:  $\phi R_{n\_tr} = 0.75 \cdot F_u \cdot t \cdot \text{Weld Length} = 292.5 \text{ K}$  (Equation J4-2)

Capacity:  $\phi R_{n1} = \text{Min}(\phi R_{n\_ty}, \phi R_{n\_tr}) = 270 \text{ K}$

Support:  $F_y = 7200 \text{ Ksf}$   $F_u = 9360 \text{ Ksf}$   $t = 0.065 \text{ ft}$

Shear Yield:  $\phi R_{n\_vy} = 1.0 \cdot 0.6 \cdot F_y \cdot t \cdot \text{Weld Length} \cdot 2 = 280.8 \text{ K}$  (Equation J4-3)

Shear Rupture:  $\phi R_{n\_vr} = 0.75 \cdot 0.6 \cdot F_u \cdot t \cdot \text{Weld Length} \cdot 2 = 273.78 \text{ K}$  (Equation J4-4)

Capacity:  $\phi R_{n2} = \text{Min}(\phi R_{n\_vy}, \phi R_{n\_vr}) = 273.78 \text{ K}$

Unity =  $R_u / \text{Min}(\phi R_{n1}, \phi R_{n2}) = 175 \text{ K} / 270 \text{ K} = \mathbf{0.648}$

### Compression Buckling - Bottom Plate (AISC 360-22 E3)

Load Set: Load Set 1 Load Combination: 1.2D + 1.6L

Demand:  $R_u = 175 \text{ K}$

Plate:  $F_y = 7200 \text{ Ksf}$  Thickness = 0.0625 ft  $A_g = 0.04557 \text{ ft}^2$   $K = 0.65$   $L = 0.08333 \text{ ft}$   $r = 0.01804 \text{ ft}$   $n = 1$

Capacity:

$KL/r = 3.002$   $F_e = 4572722 \text{ Ksf}$  (Equation E3-4)  $F_{cr} = 7195.3 \text{ Ksf}$  (Equation E3-2)

$\phi R_n = \phi \cdot A_g \cdot F_{cr} \cdot n = 0.90 \cdot 0.04557 \text{ ft}^2 \cdot 7195.3 \text{ Ksf} \cdot 1 = 295.12 \text{ K}$

Unity =  $175 \text{ K} / 295.12 \text{ K} = \mathbf{0.593}$

### Load Combinations

Code	Name	Effective Equation
ASCE 7-22 LRFD	1. 1.4D	1.4D
ASCE 7-22 LRFD	2. 1.2D+1.6L+0.5Lr	1.2D + 1.6L
ASCE 7-22 LRFD	3. 1.2D+1.6Lr+L	1.2D + 0.5L

### Design Loads

Load Key	Fvu (K)	Fau (K)	Mzu (K-ft)
0	-9.8	0	-58.8
1	-42	0	-252
2	-18.9	0	-113.4